




JPP Consulting Ltd					Page 2
4, Ironstone Way Brixworth Northampton, NN3 9UD		Hogshaw Farm, Buxton 42.5 l/s 1.940ha inc 10% UC			
Date 12/07/2024 12:12 File 11024R - ATTENUATION__1...		Designed by JPB Checked by KER			
Innovyze		Source Control 2018.1.1			
<u>Summary of Results for 100 year Return Period (+40%)</u>					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
60 min Winter	98.833	0.833	45.1	903.2	O K
120 min Winter	98.943	0.943	45.1	1022.7	O K
180 min Winter	98.984	0.984	45.1	1066.9	O K
240 min Winter	98.996	0.996	45.1	1079.4	O K
360 min Winter	99.000	1.000	45.2	1083.5	O K
480 min Winter	98.983	0.983	45.1	1065.3	O K
600 min Winter	98.954	0.954	45.1	1033.9	O K
720 min Winter	98.918	0.918	45.1	995.2	O K
960 min Winter	98.830	0.830	45.1	899.8	O K
1440 min Winter	98.621	0.621	45.1	673.3	O K
2160 min Winter	98.392	0.392	45.0	425.2	O K
2880 min Winter	98.276	0.276	43.4	299.5	O K
4320 min Winter	98.219	0.219	33.1	237.2	O K
5760 min Winter	98.190	0.190	26.9	206.5	O K
7200 min Winter	98.172	0.172	22.8	186.4	O K
8640 min Winter	98.159	0.159	20.0	171.9	O K
10080 min Winter	98.148	0.148	17.8	160.9	O K
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
60 min Winter	61.875	0.0	1000.7	62	
120 min Winter	37.983	0.0	1230.0	118	
180 min Winter	28.551	0.0	1387.6	172	
240 min Winter	23.316	0.0	1511.4	222	
360 min Winter	17.526	0.0	1704.9	278	
480 min Winter	14.313	0.0	1856.8	356	
600 min Winter	12.232	0.0	1983.8	434	
720 min Winter	10.759	0.0	2094.0	508	
960 min Winter	8.754	0.0	2271.7	656	
1440 min Winter	6.545	0.0	2547.3	922	
2160 min Winter	4.894	0.0	2865.9	1256	
2880 min Winter	3.982	0.0	3108.3	1532	
4320 min Winter	2.943	0.0	3441.2	2244	
5760 min Winter	2.374	0.0	3711.0	2944	
7200 min Winter	2.010	0.0	3926.7	3680	
8640 min Winter	1.755	0.0	4111.3	4408	
10080 min Winter	1.564	0.0	4271.4	5144	
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JPP Consulting Ltd		Page 3
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<div>Model Details</div> <div>Storage is Online Cover Level (m) 100.000</div> <div>Tank or Pond Structure</div> <div>Invert Level (m) 98.000</div> <table><thead><tr><th>Depth (m)</th><th>Area (m²)</th><th>Depth (m)</th><th>Area (m²)</th><th>Depth (m)</th><th>Area (m²)</th></tr></thead><tbody><tr><td>0.000</td><td>1084.0</td><td>1.000</td><td>1084.0</td><td>1.001</td><td>0.0</td></tr></tbody></table> <div>Hydro-Brake® Optimum Outflow Control</div> <div><div>Unit Reference MD-SHE-0282-4520-1000-4520</div><div>Design Head (m)1.000</div><div>Design Flow (l/s)45.2</div><div>Flush-Flo™Calculated</div><div>ObjectiveMinimise upstream storage</div><div>ApplicationSurface</div><div>Sump AvailableYes</div><div>Diameter (mm)282</div><div>Invert Level (m)98.000</div><div>Minimum Outlet Pipe Diameter (mm)300</div><div>Suggested Manhole Diameter (mm)1800</div></div> <div><table><thead><tr><th>Control Points</th><th>Head (m)</th><th>Flow (l/s)</th></tr></thead><tbody><tr><td>Design Point (Calculated)</td><td>1.000</td><td>45.2</td></tr><tr><td>Flush-Flo™</td><td>0.432</td><td>45.1</td></tr><tr><td>Kick-Flo®</td><td>0.774</td><td>40.0</td></tr><tr><td>Mean Flow over Head Range</td><td>-</td><td>36.4</td></tr></tbody></table><p>The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated</p><table><thead><tr><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th></tr></thead><tbody><tr><td>0.100</td><td>8.8</td><td>1.200</td><td>49.3</td><td>3.000</td><td>76.8</td><td>7.000</td><td>116.1</td></tr><tr><td>0.200</td><td>29.0</td><td>1.400</td><td>53.1</td><td>3.500</td><td>82.8</td><td>7.500</td><td>120.1</td></tr><tr><td>0.300</td><td>43.9</td><td>1.600</td><td>56.7</td><td>4.000</td><td>88.4</td><td>8.000</td><td>124.0</td></tr><tr><td>0.400</td><td>45.0</td><td>1.800</td><td>60.0</td><td>4.500</td><td>93.6</td><td>8.500</td><td>127.7</td></tr><tr><td>0.500</td><td>44.8</td><td>2.000</td><td>63.1</td><td>5.000</td><td>98.5</td><td>9.000</td><td>131.3</td></tr><tr><td>0.600</td><td>44.0</td><td>2.200</td><td>66.1</td><td>5.500</td><td>103.2</td><td>9.500</td><td>134.8</td></tr><tr><td>0.800</td><td>40.6</td><td>2.400</td><td>69.0</td><td>6.000</td><td>107.7</td><td></td><td></td></tr><tr><td>1.000</td><td>45.2</td><td>2.600</td><td>71.7</td><td>6.500</td><td>112.0</td><td></td><td></td></tr></tbody></table></div>			Depth (m)	Area (m²)	Depth (m)	Area (m²)	Depth (m)	Area (m²)	0.000	1084.0	1.000	1084.0	1.001	0.0	Control Points	Head (m)	Flow (l/s)	Design Point (Calculated)	1.000	45.2	Flush-Flo™	0.432	45.1	Kick-Flo®	0.774	40.0	Mean Flow over Head Range	-	36.4	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	0.100	8.8	1.200	49.3	3.000	76.8	7.000	116.1	0.200	29.0	1.400	53.1	3.500	82.8	7.500	120.1	0.300	43.9	1.600	56.7	4.000	88.4	8.000	124.0	0.400	45.0	1.800	60.0	4.500	93.6	8.500	127.7	0.500	44.8	2.000	63.1	5.000	98.5	9.000	131.3	0.600	44.0	2.200	66.1	5.500	103.2	9.500	134.8	0.800	40.6	2.400	69.0	6.000	107.7			1.000	45.2	2.600	71.7	6.500	112.0		
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Appendix L
Overland Flows
JPP Consulting drawing no. 11024-FRA06B

General notes

All dimensions are in metres unless otherwise stated.

All levels are in metres.

This drawing is to be read in conjunction with all relevant Engineers and Architect's drawings, Specifications, Reports and Engineering Details.

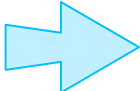
Do not scale from this drawing.


Based on Topographical Land Survey by SurveyEng Ltd, drawing number BH.TS.12 Rev C dated 26.03.2021.

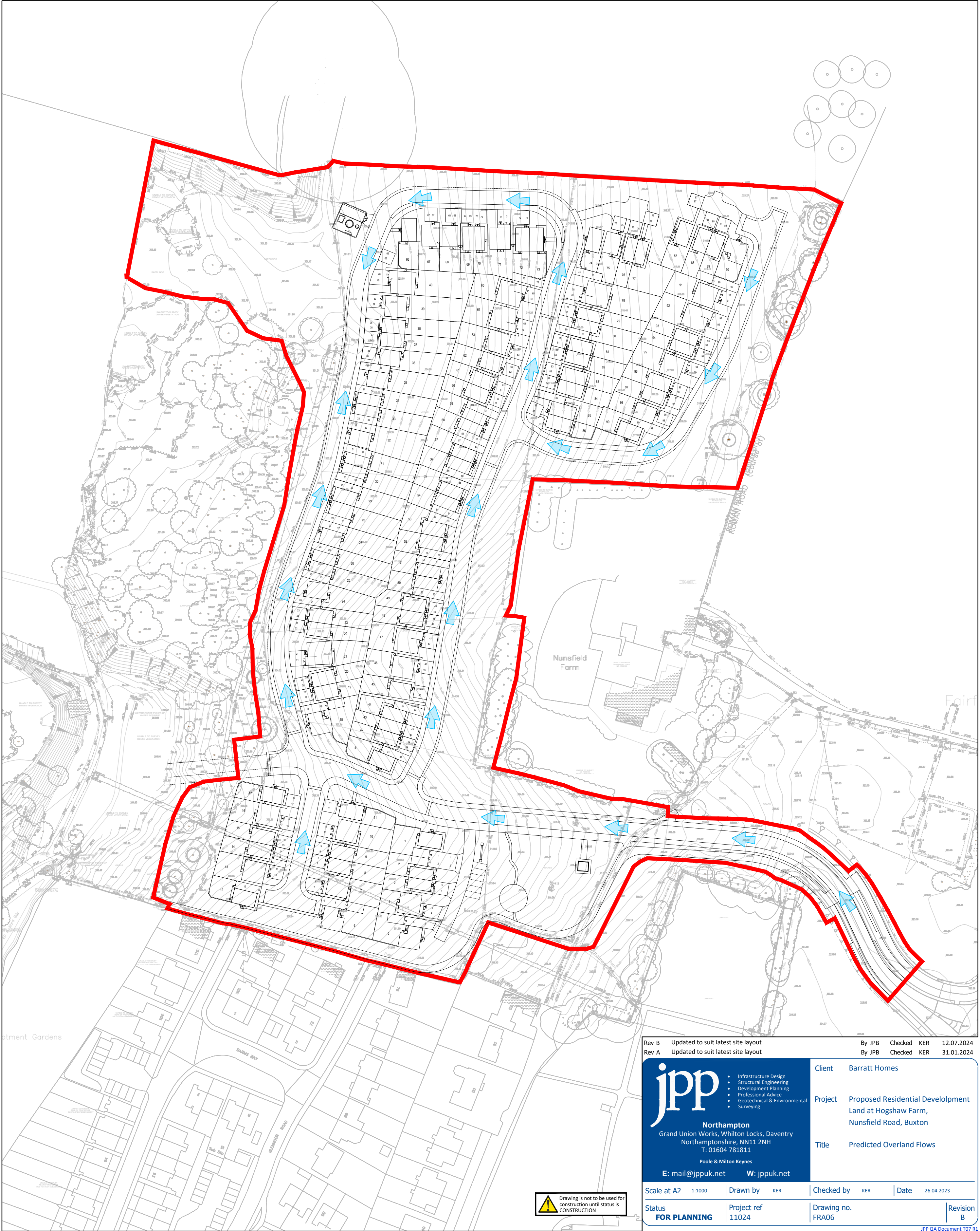
Based on Planning Layout by Barratt Homes, drawing number H8797-BAH-XX-XX-DR-UD-203001-P03 dated 28.06.2024.

Based on Drainage Layout by Barratt Homes, drawing number H8797-BAH-XX-XX-DR-CE-300001-P07 dated 01.07.2024.

Drawing Key

 Predicted Overland Flows

 Site Boundary



Rev B Updated to suit latest site layout

Rev A Updated to suit latest site layout

By JPB

By JPB

Checked KER

Checked KER

12.07.2024

31.01.2024

jpp

Northampton

Grand Union Works, Whilton Locks, Daventry

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Poole & Milton Keynes

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Client

Barratt Homes

Project

Proposed Residential Development

Land at Hogshaw Farm,

Nunsfield Road, Buxton

Title

Predicted Overland Flows

Scale at A2 1:1000

Drawn by KER

Checked by KER

Date 26.04.2023

Status

FOR PLANNING

Project ref


11024

Drawing no.

FRA06

Revision

B

 Drawing is not to be used for construction until status is CONSTRUCTION



Appendix M

Severn Trent Water Pre-Development Enquiry

WONDERFUL ON TAP



Severn Trent Water Ltd
Leicester Water Centre
Gorse Hill
Anstey
Leicester
LE7 7GU

Contact: Emma Nowak
Mob: 07970361864

Email:
Network.Solutions@SevernTrent.co.uk

Our ref: 1080322

JPP Consulting Ltd,
4 Ironstone Way,
Brixworth,
Northampton,
NN6 9UD.

FAO: Katherine Rose

20th April 2023

Dear Katherine,

Proposed Residential Development (116 Residential Dwellings) at: Land at Hogshaw Farm, Nunsfield Road, Buxton

X: 406585 / Y: 374410

I refer to your Development Enquiry Request submitted in respect of the above site. Please find enclosed the sewer records that are included in the fee together with the Supplementary Guidance Notes (SGN) referred to below.

Public Sewers in Site – Required Protection

There are no public sewers crossing the site.

Please Note: On 1st October 2011 many private sewers were transferred into the ownership of Severn Trent Water as public sewers, where two or more properties in separate ownership are served by those sewers. Most of these former private sewers will not be shown on the public sewer records, therefore a full site survey should be carried out prior to any layout design or construction works to identify where these sewers may be and to avoid later delays and possible added costs.

Foul Water Drainage

The most convenient point of connection for gravity foul flows from the site is at the 150mm public foul sewer southwest of the site at the junction of Barms Way & Nunsfield Road. A gravity connection for an additional 116 dwellings (1.8 l/s 2DWF) would be acceptable at this sewer at a new or existing manhole (5203) subject to formal 106 approval.

Please note that a gravity connection should be viable for the whole site, however should after investigations, a pumped

connection be required for any part of the site, please notify us as soon as possible further modelling may be required to understand the impact of increased flows in the foul network.

Surface Water Drainage

Under the terms of Section H of the Building Regulations 2000, the disposal of surface water by means of soakaways should be considered as the primary method. If this is not practical a connection to the watercourse southeast of the site would be acceptable subject to LLFA approval. In addition, other sustainable drainage methods should also be explored before a discharge to the public sewerage system is considered.

If these are found to be unsuitable, satisfactory evidence will need to be submitted. The evidence should be either percolation test results or by the submission of a statement from the SI consultant (extract or a supplementary letter).

The site drainage should be discussed with the Local Lead Flood Authority with a view to implement suitable SuDS techniques to land soakaways or other land drainage systems prior to any consideration of discharges to public sewers being accepted. Any discharge rate to a watercourse or drainage ditch will be determined by the LLFA.

Should soakaways / SuDs prove not to be viable, the Nun Brook runs west of the proposed development. A connection to the watercourse should be pursued but may require the crossing of third-party land. Please note, that if you cannot gain consent from the landowner, a S98 Sewer Requisition may be required. For your information, STW have the right to lay sewers through 3rd party land even unregistered land, however, STW do not have the right to discharge to a watercourse/ditch. In order to discharge to a watercourse we require a Deed Of Grant Of Easement (DOGOE) from the watercourse owner. Please liaise with the LLFA as statutory consultee in the planning process to agree flow rates at this location.

No impact on the public network.

New Connections

For any new connections including the use, reuse and indirect to the public sewerage system, the developer will need to submit Section 106 application. Our Developer Services department are responsible for handling all such enquiries and applications. To contact them for an application form and associated guidance notes, please call 0800 707 6600, email new.connections@severntrent.co.uk or download from www.stwater.co.uk

Please quote the above reference number in any future correspondence (including e-mails) with STW Limited. Please send **all correspondence** to the network.solutions@severntrent.co.uk email inbox address, a response will be made within 15 days.

If you require a VAT receipt for the application fee please email MISCINCOME.NC@SEVERNTRENT.CO.UK quoting the above Reference Number.

Please note that Developer Enquiry responses are only valid for 6 months from the date of this letter.

Yours sincerely,

A handwritten signature in black ink, appearing to read "Emma Nowak".

Emma Nowak.
Senior Evaluation Technician
Network Solutions
Developer Services



Appendix N
Hydraulic Modelling Report
JBA Technical Note 2023s1142

TECHNICAL NOTE

JBA Project Code
Contract
Client
Date
Author
Reviewer / Sign-off
Subject

2023s1142
Hydraulic Modelling at Hogshaw Farm
Barratt Homes Manchester
January 2024
Ella Albrighton BSc
Amy Evans BSc MCIWEM C.WEM
Hydraulic Modelling at Hogshaw Farm, Buxton

JBA
consulting

1 Introduction

1.1 Terms of reference

JBA Consulting were commissioned by JPP Consulting Ltd on behalf of Barratt Homes Manchester to undertake a fluvial hydraulic modelling study in order to support their planning application for a proposed development at their site at Hogshaw Farm, Buxton.

The Nun Brook, which becomes an Environment Agency Main River downstream of the site, flows in a southerly direction through the north-western part of the site. The watercourse is culverted during this reach. The culvert has a diameter of 1.22m. The location of the site and the Nun Brook are included within Figure 1-1 below.

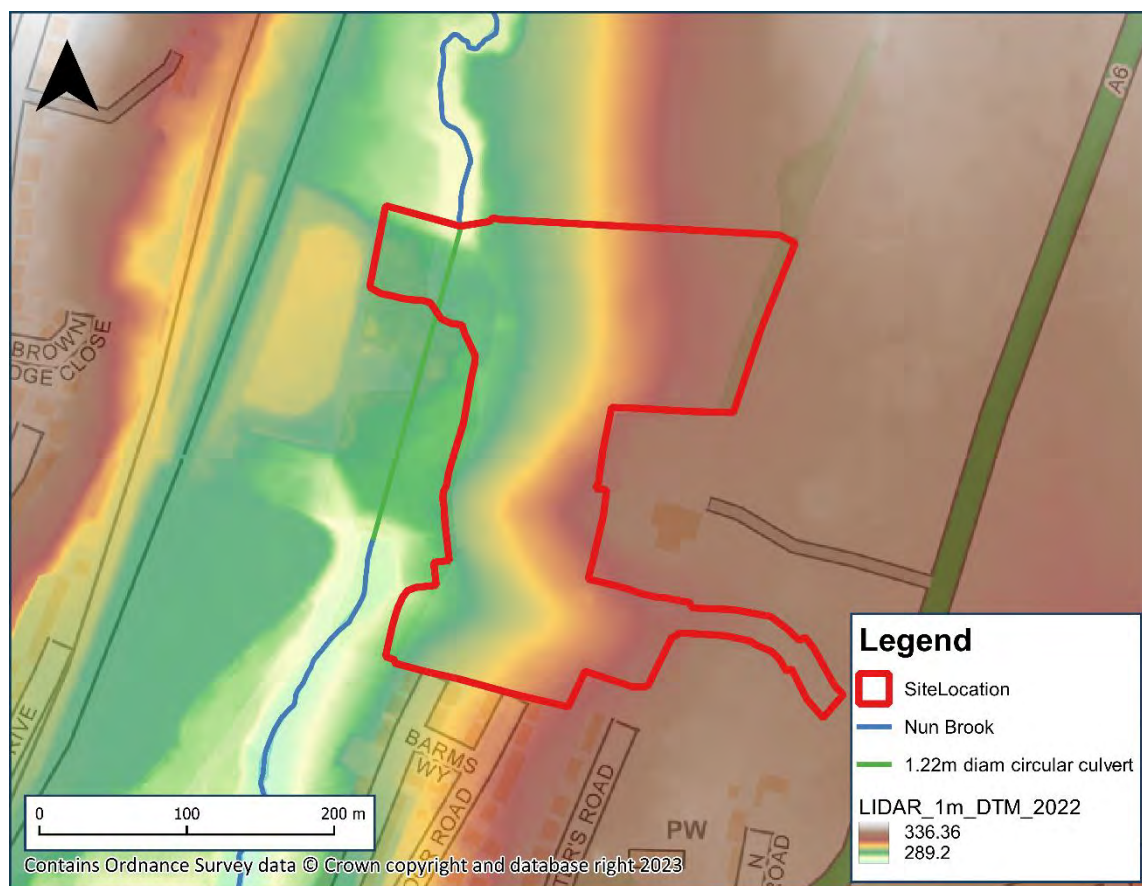


Figure 1-1 - Site Location

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2 Baseline Hydraulic modelling

2.1 Modelling Approach

A license to re-use the 1D-2D ESTRY-TUFLOW 2022 Hogshaw and Nun Brook hydraulic model was obtained from the Environment Agency for use in this study. The hydrology inflows, calculated in 2022 as part of the original modelling, were retained as these still reflect best practises.

The extent of the existing model in relation to the site is shown within Figure 2-1 below.

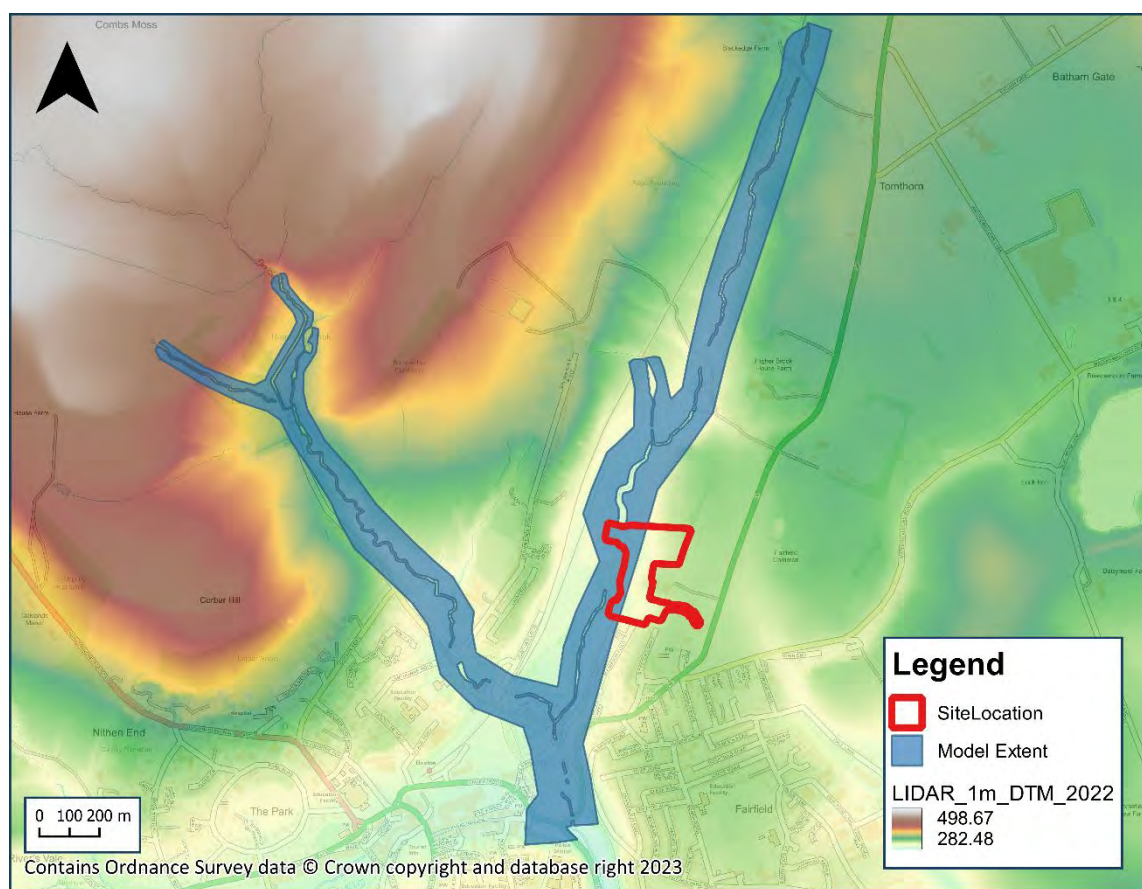


Figure 2-1: Existing model extent

New topographic survey data (included within Appendix A) representing the current ground levels within the site was integrated within the model.

The updated and refined baseline model was run with TUFLOW version 2023-03-AA-iDP-w64 for the 30-year, 100-year, 100-year with climate change (29%) and 1000-year fluvial flood events.

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2.2 Baseline model results

2.2.1 Baseline flood extents

The baseline flood extents during the 30-year, 100-year and 1000-year events are shown in Figure 2-2 below. Figure 2-3 shows the impact of climate change on the 100-year flood extents.

This shows that only the north-western area of the site is included with the maximum flood extent during all of the modelled flood extents.

Figure 2-3 shows that, when 29% climate change is applied to the 100-year event, there is no impact on flood extents within the site. Upstream of the site, there is a slight increase in flood extent.

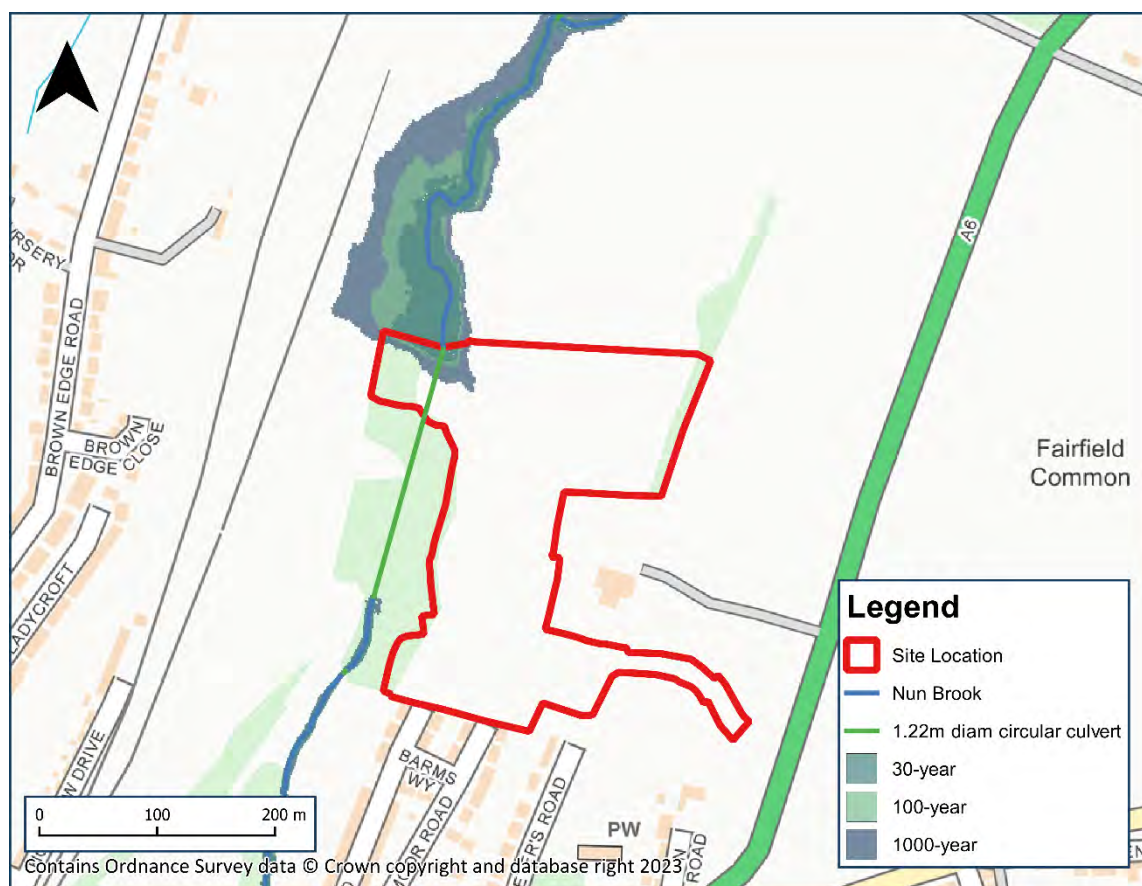


Figure 2-2: Baseline flood extents

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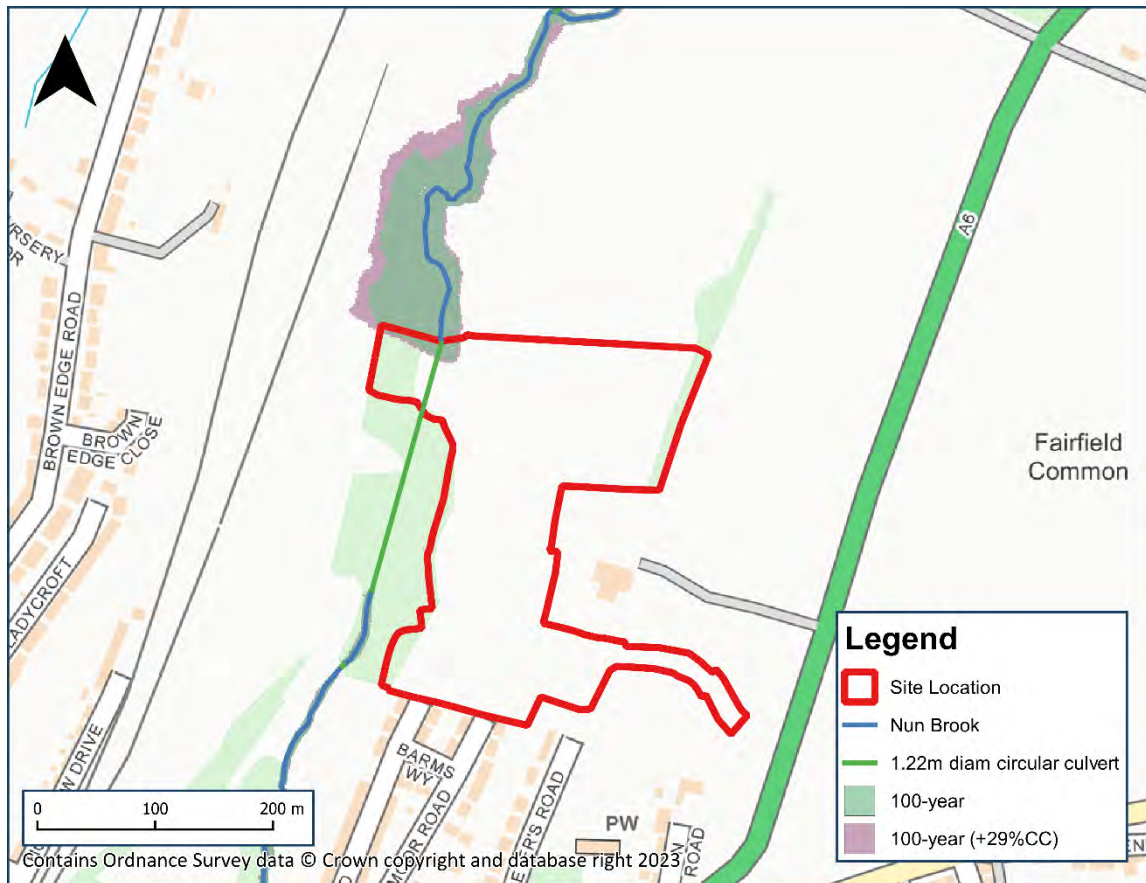


Figure 2-3: Baseline flood extent with climate change

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2.2.2 Baseline flood depths

The flood depths within the site during the 100-year with 29% climate change flood event (i.e. the design event) are shown within Figure 2-4.

This shows that within the site, the maximum flood depths are upstream of the 1.22m diameter culvert inlet. The maximum flood depth in this area is 4.02m, within the area of low elevation surrounding the channel.

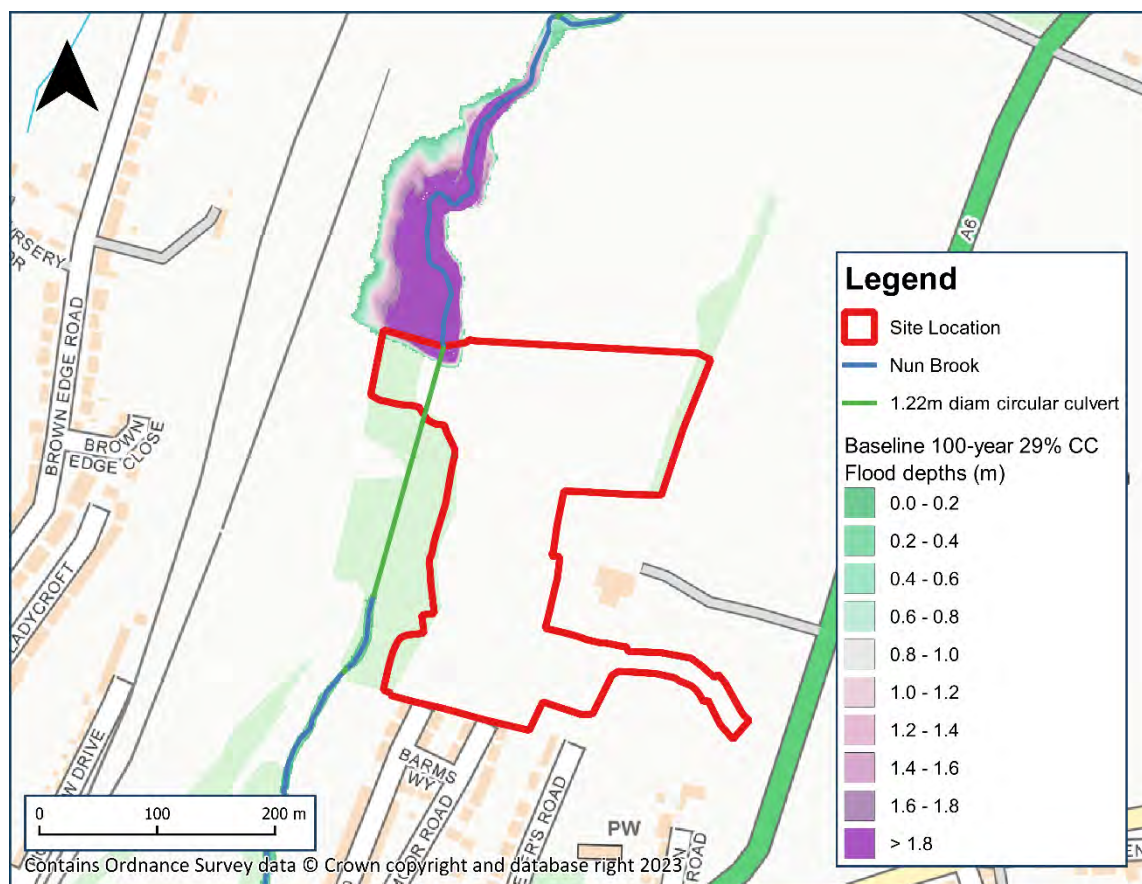


Figure 2-4: Baseline flood depths

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2.2.3 Baseline water levels

The water levels during the baseline 100-year with 29% climate change are shown in Figure 2-5. During the baseline 100-year with 29% climate change event, the peak water level onsite and at the inlet of the culvert is 299.90m AOD.

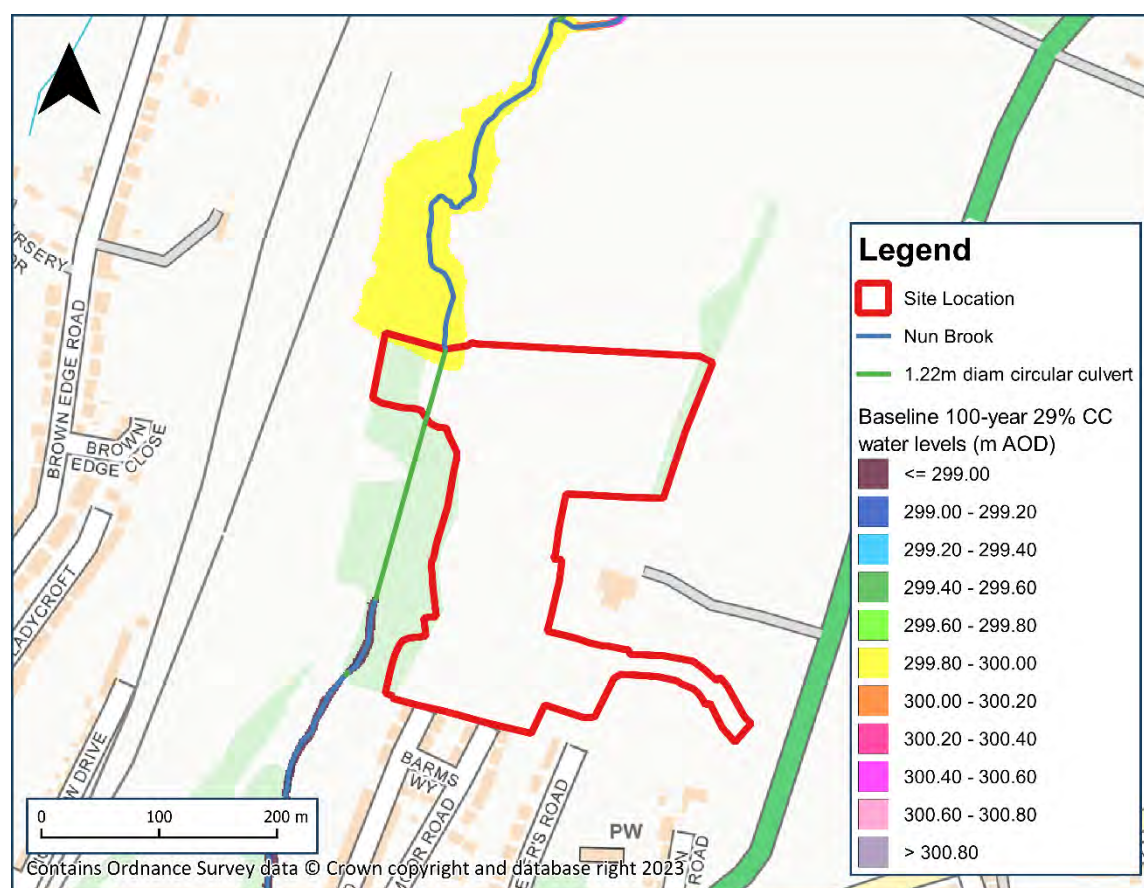


Figure 2-5: Baseline water levels

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2.2.4 Baseline hazard to people

The fluvial hazard-to-people rating during a 100-year with 29% climate change flood scenario has been mapped using the formula as suggested in Defra's FD2321/TR2 "Flood Risk to People". The different hazard categories are shown in Table 2-1 and the hazard classification to the site during the 100-year with 29% climate change scenario is shown in Figure 2-6.

Table 2-1 Flood Risk to People Hazard to People

Flood hazard rating depth $\times (\text{velocity} + 0.5) + \text{DF}$	Level of Flood Hazard Description	Class label
<0.75	Low	Caution "Flood zone with shallow flowing water or deep standing water"
0.75 to 1.25	Moderate	Dangerous for some (i.e. Children) "Danger: flood zone with deep or fast flowing water"
1.25 to 2.00	Significant	Danger for most "Danger: flood zone with deep fast flowing water"
>2.00	Extreme	Dangerous for all "Extreme danger: flood zone with deep fast flowing water"
Using the hazard equation $HR = d \times (v + 0.5) + DF$ Where d = depth of flooding (m) v = velocity of floodwaters (m/sec) DF = debris factor		

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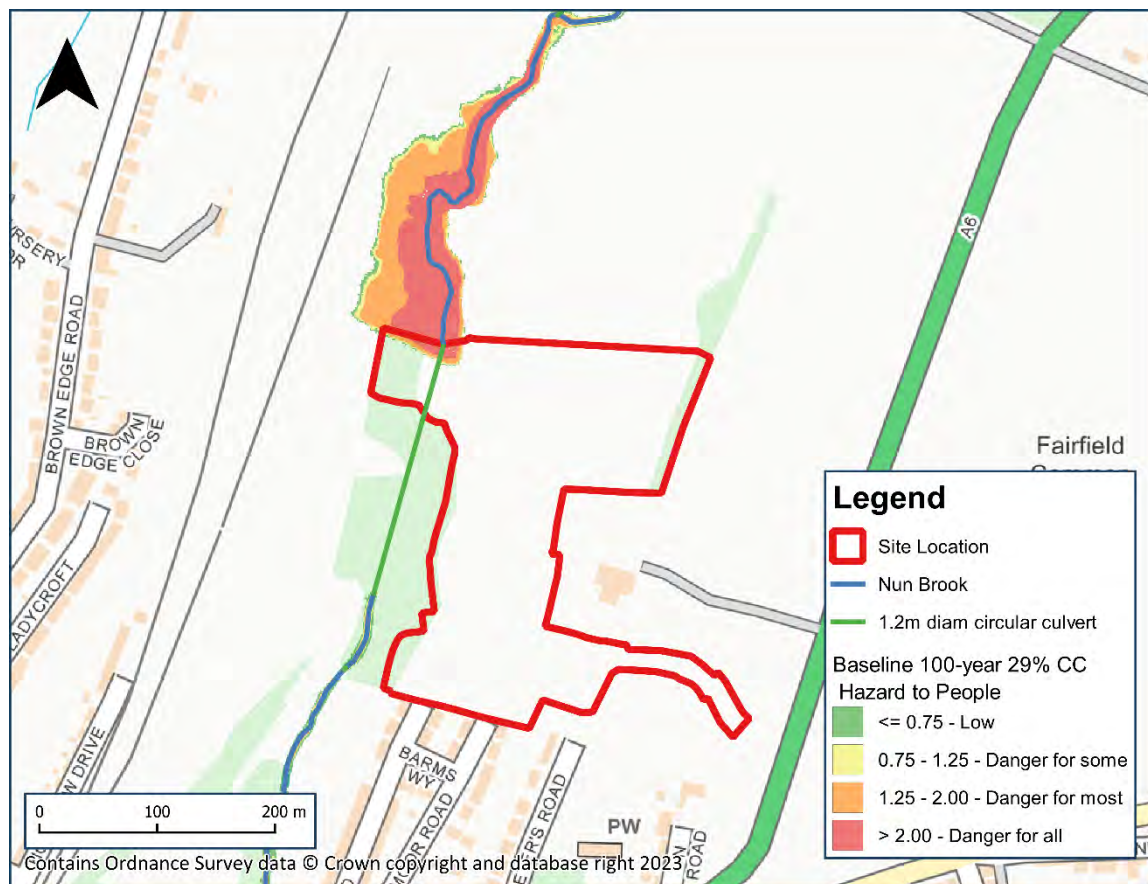


Figure 2-6: Baseline hazard to people

Figure 2-6 shows that during the baseline 100-year with 29% climate change event the north-western area of the site is partially within the 'danger for all' hazard category. However, this is primarily the channel and immediate floodplain of the Nun Brook.

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2.3 Blockage analysis

The residual risk of a blockage at the inlet of the 1.22m diameter culvert has been considered as part of this study. In order to represent the worst-case scenario a 100% blockage at the structure has been modelled. Figure 2-7 shows the maximum flood extent during the 100% blockage at the structure during a 100-year with 29% climate change flood event. This shows that the flood extent is larger during this event where flows build up behind the inlet before eventually flowing overland to the downstream culvert extent where the Nun Brook becomes open channel.

Figure 2-8 shows the peak water levels associated with this residual event.

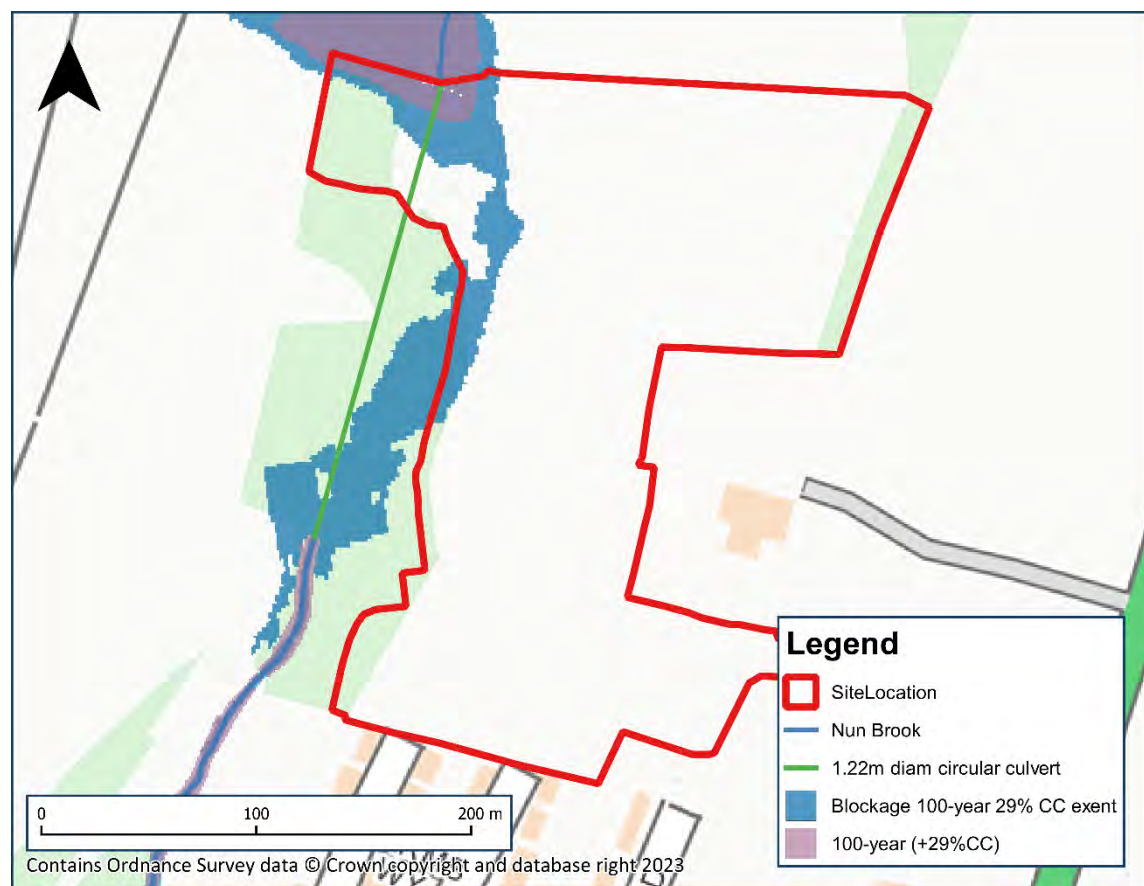


Figure 2-7: Blockage extent